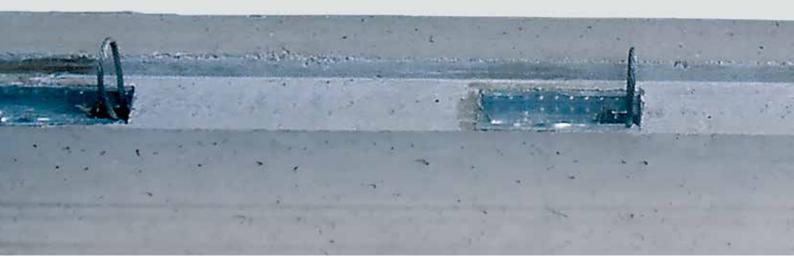


# PFEIFER VS<sup>®</sup> connecting loops

Technical Manual



# **PFEIFER VS<sup>®</sup> connecting loops**

The PFEIFER VS<sup>®</sup>-Boxes are designed for economical connections of wall-type pre-cast concrete elements and for the connection of columns and walls.

With the PFEIFER VS<sup>®</sup>-Boxes, wall joints can be produced inexpensively, easily and safely.

The products consist of a sturdy sheet steel box in which one or two fold-out, flexible wire rope connection loops are located. The coloured clip within the boxes identifies the respective length of the integrated wire rope loops resp. the type of box. Only boxes with clips of the same colour can be combined.





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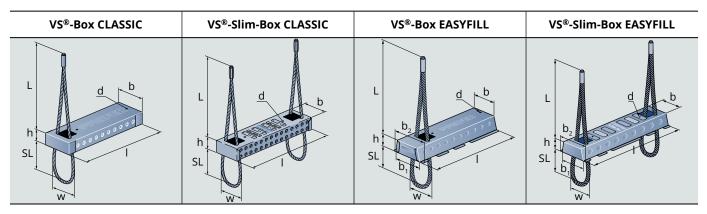
# 1. Product description

## 1.1 Types



Box types with single or double loop

## 1.2 Product dimensions



Туре	Ref. no.		Dimensions			Colour clip	Packing unit	Weight approx.					
		b	b <sub>1</sub>	b <sub>2</sub>	1	h	L	SL	w	d	- •		
[mm] [mm] [mm] [mm] [mm] [mm] [mm] [mm]									[pieces]	[kg/pcs]			
VS <sup>®</sup> -Box CLASSIC	389146	50	-	-	160	20	192	80	60	3	black	110	0,21
VS <sup>®</sup> -Slim-Box CLASSIC	410244	50	-	-	180	20	192	80	60	3	black	80	0,40
VS <sup>®</sup> -Box EASYFILL	325523	45	50	60	160	20	180	80	55	3	black	800	0,21
VS <sup>®</sup> -Slim-Box EASYFILL	309578	45	50	60	200	20	204	80	55	3	blue	400	0,36

Ø-Rope: 6,0 mm

## 1.3 Product properties

#### Table 1

	VS <sup>®</sup> -Box CLASSIC	VS <sup>®</sup> -Slim-Box CLASSIC	VS <sup>®</sup> -Box EASYFILL	VS <sup>®</sup> -Slim-Box EASYFILL							
Box material	Steel sheet, galvanized										
Rope material		Round strand r	ope, galvanized								
Rope diameter	6 mm										

# 2. Application condition

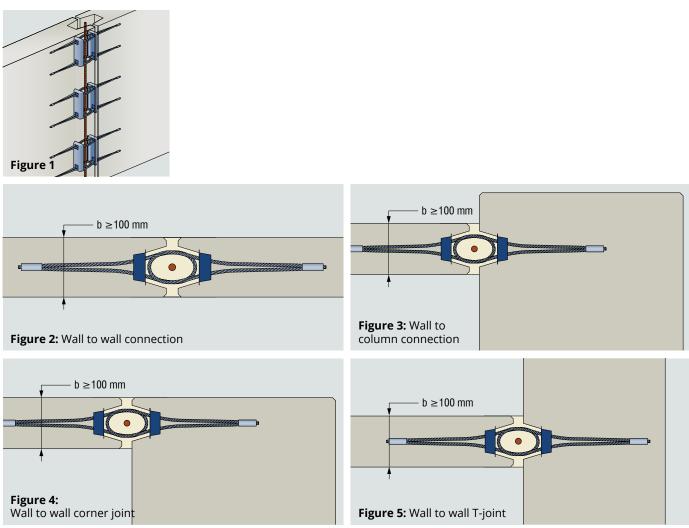
## 2.1 Intended use

The VS<sup>®</sup>-Boxes may be used to connect precast concrete walls to each other and to precast concrete columns. **Table 2** 

VS <sup>®</sup> -Box CLASSIC	VS <sup>®</sup> -Slim-Box CLASSIC	VS <sup>®</sup> -Box EASYFILL	VS <sup>®</sup> -Slim-Box EASYFILL
b	J.		
Flowable	e grout 🗸	Flowabl	e grout 🗸
Thixotropic-pl	lastic mortar X	Thixotropic-p	lastic mortar 🗸

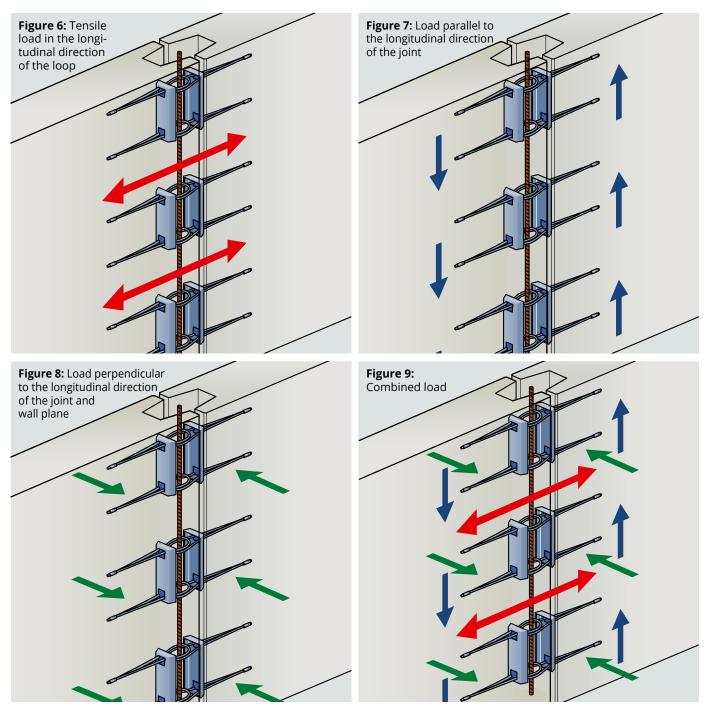
## 2.2 Application range

The boxes are installed "recessed", which means that both opposite retaining boxes are installed set back from the external edges of the precast element.



## 2.3 Load transmission possibilities

The system can transmit tensile forces, parallel shear forces and perpendicular shear forces, both individually and in combination. The load directions are illustrated taking the double-loop boxes as an example.



# 3. Structural element properties

# 3.1 Dimensions and reinforcement

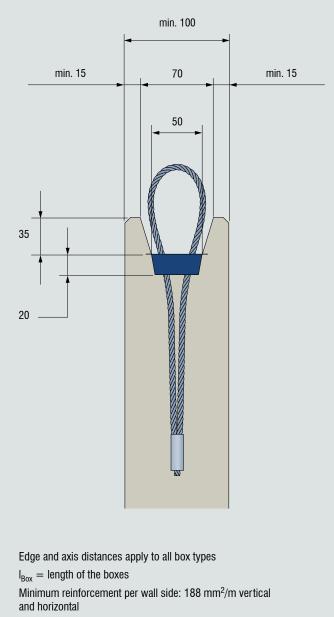
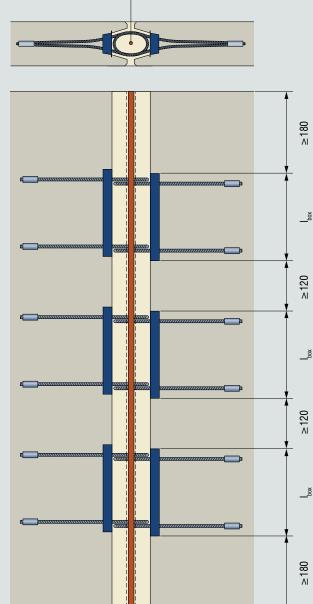
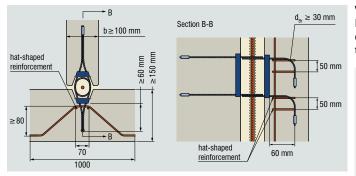


Figure 10: Geometry of the deepened joint and reinforcement



Longitudinal reinforcement B500A/B, Ø12 mm

## 3.2 Additional notes



Wall to wall T-joint - detail with bent wire rope loops

VS<sup>®</sup>-Boxes are to be installed with an appropriate concrete cover. Proof of the required concrete cover is to be provided in accordance with EN 1992-1-1, section 4. The required exposure class is to be selected according to the environmental requirements.

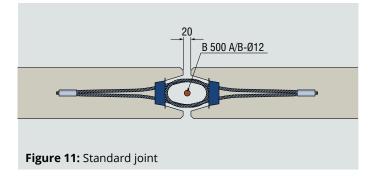


#### Note about bending anchoring loops:

When the elements have small dimensions, the anchoring loop of the VS rope loop box can be bent. With T-joints, the thickness of the abutted wall can therefore be reduced to 150 mm. The working load limits (Tables 6 to 10) of the bent rope loop box are to be limited according to the specification of ETA-22/0224. Test series have shown that an additional "hat-shaped" reinforcement is necessary for anchoring the regular loads.

# 4. Structural element connection

## 4.1 Joint geometry



## 4.2 Joint filling mortar

#### 4.2.1 Notes and advantages

The joint casting grout consists of cement, mineral aggregate and water and, if necessary, concrete additives and/or concrete admixtures.

Production:

- Delivered to the installation site as prefabricated dry mixture (bagged product) and mixed directly on site with water (before installation)
- Delivered to the installation site as finished product, after being mixed in a concrete plant on the basis of defined properties

#### Table 3

Charac	teristics							
Flowable grout	Thixotropic-plastic mortar							
Usab	le with							
Work	ability							
High	Medium-high							
Consi	onsistence							
Fluid	Viscous							
Joint	filling							
Optimal	Extra care: avoid empty spaces							
Fillin	g tools							
Standard	Extra tools i. e.: nozzle							
Joint	closing							
Closure of the joint on both sides, e.g.: shuttering board	On one side for short joint lengths i. e.: strong duct tape							
Worke	rs' skills							
Standard	Instructed personnel only (induction is necessary in machine and grouting technology, mortar systems, etc.)							

## 4.2.2 Requirements

Definition of the properties of the joint casting grout:

#### Table 4

Properties		Flowable grout	Thixotropic-plastic mortar				
Maximum grain size		≤ 8	mm				
Consistency		EN 206, Table 5 ≥ F5 EN 206, Table 6 ≥ SF1 slump-flow – EN 13395-2 ≥ 550 mm	slump-diameter – EN 13395-1 ≥ 130 mm				
Expansion after 24 hour	s according to EN 445	≥ +0,1	≥ +0,1 Vol-%				
Shrinkage value	EN 12390-16	average value: ε <sub>m.91</sub> ≤ 1,5 ‰					
after 91 days	EN 12617-4	single value:	ε <sub>i,91</sub> ≤ 2,0 ‰				
Compressive strength	EN 206, cube	$f_{c,cube,g} \ge 4$	10 N/mm²				
after 28 days	EN 196-1, prism	$f_{c,prism,g} \ge f_{c}$	<sub>.cube,g</sub> / 0,85				

# 5. Installation instructions

## 5.1 Manufacture of the structural elements

Most wall elements are concreted on formwork tables. The following work steps are necessary (see figures 15 and 16):

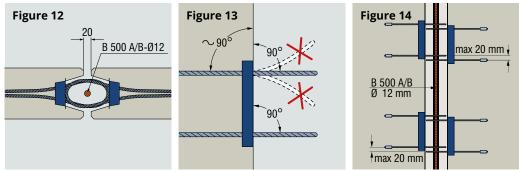
- Fastening of a trapezoidal-shaped recess strip at the front-sided vertical end of the wall element
- Straight threading of the rope end into the reinforcement and fixing 90° to the joint
- Nailing or gluing of the boxes to the formwork



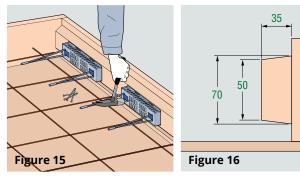
It must be ensured that the boxes are always located at the same height (offset  $\leq$  20 mm).

### Tolerances

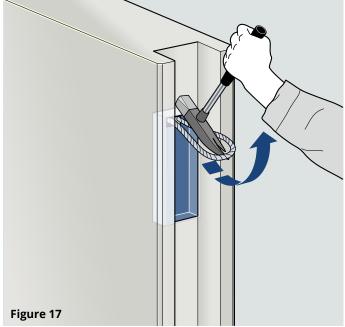
The VS<sup>®</sup> Slim Box connection acts as an overlapping joint. For that reason, the loops must each lie above one another within certain vertical and horizontal tolerances.



#### Installation

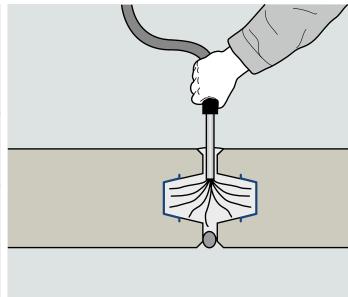


## 5.2 Connection of the structural elements



#### Before connecting the precast elements:

- Remove the covering adhesive tape
- Release the wire rope loop from the fixing
- Fold out to the intended position (90° to the joint)
- Remove dirt or grease from the joint
- Check the position of the wire rope loop
- Thread in longitudinal reinforcement B500A/B, Ø 12 mm



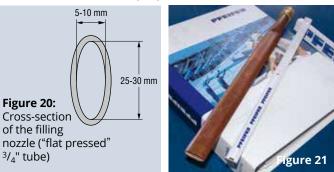
#### Figure 18

#### **Connection of the precast elements:**

- Complete closure of one side of the joint (foam, rubber hose, shuttering board)
- Fill the open joint side evenly and continuously from bottom to top

#### Nozzle making

The user can make the filling nozzle from commercially available 22 mm  $\binom{3}{4}$ ") copper heating pipe and attach it to the pump hose with the aid of a solder fitting (Figures 20 and 21).





this information only concerns the introduction of the thixotropic/plastic material into the joint!

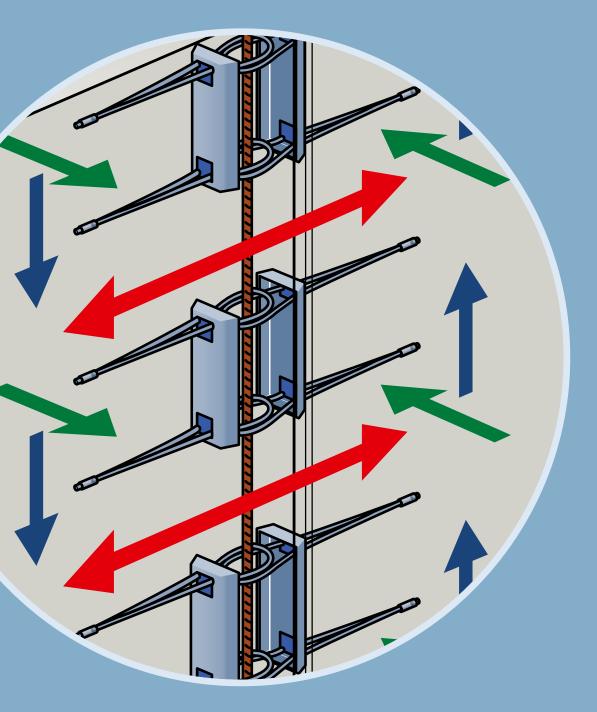
## Caution:

**Do not constrict the filling space.** If pre-compressed strips are to be pressed into the side joints without affecting the casting space, the effective lateral concrete cover of the rail and the wire rope loop is reduced. This must also be taken into account by the planners in the dimensioning.

#### After joining the precast elements: • Smooth the joint with a trowel

Figure 19

# Dimensioning



# 6. Dimensioning and planning

The precast concrete elements being connected must be construed by a responsible planner in a minimum concrete grade of C25/30 in accordance with EN 1992-1-1. The connection made with the PFEIFER-VS<sup>®</sup> rope loop boxes is considered to be a reinforced joint with design resistances for tensile and transverse shear forces. The corresponding design resistances are based on a detailed dimensioning concept and are listed in the tables in chapter 6.4. The dimensioning concept was created on the basis of an external expert's report and the design resistances confirmed by an independent expert opinion.

### 6.1 Dimensioning concept

The tensile and shear resistances of the PFEIFER-VS<sup>®</sup> rope loop boxes are determined by calculation on the basis of a dimensioning concept. This concept is based on different load-bearing models that map the working load limits of the various elements and components of a wire rope loop connection with grouting box (e.g. anchoring in concrete as well as shear models and strut and tie models). The resistances of the individual components are determined and overlaid, resulting in the working load limit of the complete system. The calculated working load limits are calibrated and verified on the basis of structural element tests.

#### Load case: assembly

No loads can be borne by the PFEIFER-VS<sup>®</sup> rope loop boxes during the assembly of the precast concrete elements and without filled element joints. The stability of the structural elements must be ensured, for example, by temporary props (e. g. PFEIFER MoFi fastener for push-pull props).

#### Load case: final condition

The PFEIFER-VS<sup>®</sup> rope loop boxes are fully load-bearing after the target strength of the joint mortar is reached. The loads according to chapter 6.4 can thus be borne by the rope loop boxes.

Crack widths as a result of constraining forces must be limited through planning to 0.3 mm.

## 6.2 Additional information on the tables

The design resistances in chapter 6.4 are to be selected in relation to the box type, the compressive strength of the structural element, the compressive strength of the joint mortar and the thickness of the structural element. Both the strength grades and the correlating cube and prism compressive strengths according to EN 206:2013, section 8.2.1.3.2 are specified. Selection can be made according to the manufacturer's specifications.

When dimensioning the connection, the evidence for each load direction must be kept individually. When overlaying tensile and transversal shear loads, note that the tensile forces arising from the shear forces must be taken into account with an externally acting tensile force (see interaction condition, chapter 6.5).

## 6.3 Safety factors

The dimensioning values in the ultimate limit state listed in chapter 6.4 were determined taking into account the following safety factors:

Table 5: Safety factors for ULS dimensioning		
Partial safety factor for concrete in general:	Υ <sub>c</sub>	= 1,50
Partial safety factor for concrete cone failure:	Yc	= 1,50
Partial safety factor for pull-out:	Yc	= 1,50
Partial safety factor for pry-out:	Yc	= 1,50
Partial safety factor for concrete with transversal shear load perpendicular to the joint:	Yc	= 1,50
Factor for taking into account long-term influences on the concrete compressive strength:	a <sub>cc</sub>	= 0,85
Factor for taking into account long-term influences on the concrete tensile strength:	a <sub>ct</sub>	= 0,85
Partial safety factor for round strand rope:	Y <sub>s,Rope</sub>	= 1,50
Partial safety factor for reinforcing steel:	Y <sub>s,B</sub>	= 1,20
System reduction factor for EASYFILL <i>single rope</i> with load N <sub>Rd</sub> /V <sub>II,Rd</sub> :	η <sub>E1</sub>	= 0,85
System reduction factor for EASYFILL <i>double rope</i> with load N <sub>Rd</sub> /V <sub>II,Rd</sub> :	$\eta_{E2}$	= 0,95
System reduction factor for CLASSIC <i>single rope</i> with load N <sub>Rd</sub> /V <sub>II,Rd</sub> :	η <sub>c1</sub>	= 0,85
System reduction factor for CLASSIC double rope with load $N_{Rd}/V_{II,Rd}$ :	$\eta_{C2}$	= 0,95
System reduction factor for $N_{Rd}/V_{II,Rd}$ : when using free-flowing grouts	ην	= 1,00
System reduction factor for $N_{Rd}/V_{II,Rd}$ : when using plastic (thixotropic) joint mortars	η <sub>P</sub>	= 0,90

The system reduction factors are used to map influences from scaling effects, variances, imperfections, eccentricities or environmental influences.

## 6.4 Dimensioning Tables

Grout **Precast concrete** Average prism Average cube Compressive compressive compressive strength C25/30 C30/37 C35/45 C40/50 strength strength class f<sub>c,m,prism,28d</sub> [N/mm<sup>2</sup>] V<sub>Rd,II</sub> [kN] N<sub>Rd</sub> V<sub>Rd,II</sub> fc,m,cube,28d N<sub>Rd</sub> V<sub>Rd,II</sub> N<sub>Rd</sub> V<sub>Rd,II</sub> N<sub>Rd</sub>  $[N/mm^2]$ [-] [kN] [kN] [kN] [kN] [kN] [kN] [kN] Wall thickness h = 100 mm ≥ 49 ≥ 42 C30/37 7,3 11,4 7,3 11,4 7,3 11,4 7,3 11,4 Grout ≥ 65 ≥ 55 C40/50 12,7 13,8 9,8 14,7 9,8 15,0 8,7 9,5 C50/60 13,1 14,1 ≥ 76 ≥ 65 10,3 15,0 11,0 15,9 Wall thickness h ≥ 120 mm ≥ 49 ≥ 42 C30/37 7,3 11,4 11,4 11,4 7,3 11,4 7,3 7,3 ≥ 65 ≥ 55 C40/50 12,9 9,8 15,0 9,8 15,0 9,8 15,0 8,9 ≥ 76 ≥ 65 C50/60 13,3 10,6 15,5 17,6 12,2 18,4 12,2

Table 6: VS<sup>®</sup>-Box EASYFILL 1-Rope – Design resistance per pair of boxes

Table 7: VS®-Slim-Box EASYFILL 2-Ropes – Design resistance per pair of boxes

		Grout					Precast	concrete					
	Average prism compressive strength	Average cube compressive strength	Compressive strength class	C25	5/30	C30	C30/37		5/45	C40	0/50		
	<b>f<sub>c,m,prism,28d</sub></b> [N/mm²]	<b>f<sub>c,m,cube,28d</sub></b> [N/mm²]	- [-]	N <sub>Rd</sub> [kN]	V <sub>Rd,II</sub> [kN]	N <sub>Rd</sub> [kN]	V <sub>Rd,II</sub> [kN]	N <sub>Rd</sub> [kN]	V <sub>Rd,II</sub> [kN]	N <sub>Rd</sub> [kN]	V <sub>Rd,II</sub> [kN]		
	Wall thickness h = 100 mm												
	≥ 49	≥ 42	C30/37		25,3		25,3		25,3		25,3		
	≥ 65	≥ 55	C40/50	12,5	26,9	13,7	29,2	14,8	31,1	15,8	32,8		
	≥ 76	≥ 65	C50/60		26,9		29,5		31,8		33,8		
			Wall	thicknes	s h = 120	mm							
	≥ 49	≥ 42	C30/37		25,3		25,3	16,4	25,3	16,4	25,3		
	≥ 65	≥ 55	C40/50	15,0	27,4	16,4	31,9	17,8	33,6	19,0	33,6		
1no	≥ 76	≥ 65	C50/60		27,4		32,9	17,0	37,2	19,0	39,5		
5			Wa	ndstärke	h = 150 r	nm							
	≥ 49	≥ 42	C30/37	16,4	25,3	16,4	25,3	16,4	25,3	16,4	25,3		
	≥ 65	≥ 55	C40/50	18,8	27,4	20,5	31,9	21,9	33,6	21,9	33,6		
	≥ 76	≥ 65	C50/60	10,0	27,4	20,5	32,9	22,2	37,2	23,7	41,1		
			Wall	thicknes	s h = 180	mm							
	≥ 49	≥ 42	C30/37	16,4	25,3	16,4	25,3	16,4	25,3	16,4	25,3		
	≥ 65	≥ 55	C40/50	19,8	27,4	21,9	31,9	21,9	33,6	21,9	33,6		
	≥ 76	≥ 65	C50/60	19,0	27,4	23,8	32,9	26,6	37,2	27,4	41,1		
	Wall thickness h ≥ 200 mm												
	≥ 49	≥ 42	C30/37	16,4	25,3	16,4	25,3	16,4	25,3	16,4	25,3		
	≥ 65	≥ 55	C40/50	19,8	27,4	21,9	31,9	21,9	33,6	21,9	33,6		
	≥ 76	≥ 65	C50/60	, , , , , , , , , , , , , , , , , , , ,		23,8	32,9	26,6	37,2	27,4	41,1		

The resistances apply when using free-flowing grout. No plastic (thixotropic) joint filling mortars may be used with the box types VS<sup>®-</sup>Box CLASSIC and VS<sup>®</sup>-Slim-Box CLASSIC.

 Table 8: VS<sup>®</sup>-Box CLASSIC 1-Rope – Design resistance per pair of boxes

		Grout		Precast concrete										
	Average prism compressive strength	Average cube compressive strength	Compressive strength class	C25	5/30	C30	)/37	C35	6/45	C40/50				
	<b>f<sub>c,m,prism,28d</sub></b> [N/mm²]	<b>f<sub>c,m,cube,28d</sub></b> [N/mm²]	- [-]	N <sub>Rd</sub> [kN]	V <sub>Rd,II</sub> [kN]									
	Wall thickness h = 100 mm													
rout	≥ 49	≥ 42	C30/37	7,3	11,4	7,3	11,4	7,3	11,4	7,3	11,4			
Ğ	≥ 65	≥ 55	C40/50	9,8	15,0	9,8	15,0	9,8	15,0	9,8	15,0			
	≥ 76	≥ 65	C50/60	9,8	15,4	10,7	16,6	11,6	17,6	12,2	18,4			
			Wall	thicknes	s h ≥ 120	mm								
	≥ 49	≥ 42	C30/37	7,3	11,4	7,3	11,4	7,3	11,4	7,3	11,4			
	≥ 65	≥ 55	C40/50	9,8	15,0	9,8	15,0	9,8	15,0	9,8	15,0			
	≥ 76	≥ 65	C50/60	10,1	15,8	12,1	18,3	12,2	18,4	12,2	18,4			

#### Table 9: VS<sup>®</sup>-Slim-Box CLASSIC 2-Ropes – Design resistance per pair of boxes

		Grout					Precast	concrete	;				
	Average prism compressive strength	Average cube compressive strength	Compressive strength class	C25	5/30	C30	)/37	C35	5/45	C40	)/50		
	<b>f<sub>c,m,prism,28d</sub></b> [N/mm²]	<b>f<sub>c,m,cube,28d</sub></b> [N/mm²]	- [-]	N <sub>Rd</sub> [kN]	V <sub>Rd,II</sub> [kN]								
	Wall thickness h = 100 mm												
	≥ 49	≥ 42	C30/37		25,3		25,3	14,8	25,3		25,3		
	≥ 65	≥ 55	C40/50	12,5	32,9	13,7	33,6		33,6	15,8	33,6		
	≥ 76	≥ 65	C50/60		32,9		34,0		35,1		36,1		
			Wall	thicknes	s h = 120	mm							
	≥ 49	≥ 42	C30/37		25,3		25,3	16,4	25,3	16,4	25,3		
	≥ 65	≥ 55	C40/50	15,0	33,6	16,4	33,6	17,8	33,6	19,0	33,6		
rout	≥ 76	≥ 65	C50/60		34,6		36,0	17,0	37,2	19,0	38,5		
פֿ			Wall	thickness h = 150 mm									
	≥ 49	≥ 42	C30/37	16,4	25,3	16,4	25,3	16,4	25,3	16,4	25,3		
	≥ 65	≥ 55	C40/50	18,8	33,6	20,5	33,6	21,9	33,6	21,9	33,6		
	≥ 76	≥ 65	C50/60	10,0	34,6	20,5	38,9	22,2	40,5	23,7	41,1		
			Wall	thicknes	s h = 180	mm	,	,	1				
	≥ 49	≥ 42	C30/37	16,4	25,3	16,4	25,3	16,4	25,3	16,4	25,3		
	≥ 65	≥ 55	C40/50	21,9	33,6	21,9	33,6	21,9	33,6	21,9	33,6		
	≥ 76	≥ 65	C50/60	22,5	34,6	24,7	40,7	26,6	41,1	27,4	41,1		
			Wall	thicknes	s h ≥ 200	mm							
	≥ 49	≥ 42	C30/37	16,4	25,3	16,4	25,3	16,4	25,3	16,4	25,3		
	≥ 65	≥ 55	C40/50	21,9	33,6	21,9	33,6	21,9	33,6	21,9	33,6		
	≥ 76	≥ 65	C50/60	22,5	34,6	27,0	40,7	27,4	41,1	27,4	41,1		

The resistances apply when using free-flowing grout. Plastic (thixotropic) joint filling mortars may also be used with the box types VS<sup>®</sup>-Box EASYFILL and VS<sup>®</sup>-Slim-Box EASYFILL. In this case, the resistances specified above must be reduced by the factor 0.9.

Table 10: Vertical transverse force v<sub>Rd.L</sub> – Design resistance for all four box types per running meter

		Grout						Pr	ecast	concre	te				
	Average prism compressive strength	Average cube compressive strength	Compressive strength class	V <sub>Rd,L</sub>											
	<b>f</b> <sub>c,m,prism,28d</sub>	<b>f</b> <sub>c,m,cube,28d</sub>	-						[kN	/m]					
	[N/mm²]	[N/mm²]	[-]	C25/30	C30/37	C35/45	C40/50	C25/30	C30/37	C35/45	C40/50	C25/30	C30/37	C35/45	C40/50
				Wall t	hicknes	s h = 10	00 mm	Wall t	hicknes	is h = 12	20 mm	Wall t	hicknes	s h = 14	40 mm
	≥ 49	≥ 42	C30/37												
	≥ 65	≥ 55	C40/50	3,9	4,5	5,2	5,5	6,1	7,0	8,0	8,5	8,4	9,7	11,1	11,9
out	≥ 76	≥ 65	C50/60												
ษ				Wall t	hicknes	s h = 15	50 mm	Wall t	hicknes	is h = 16	50 mm	Wall ti	hicknes	is h = 18	80 mm
	≥ 49	≥ 42	C30/37						11,0 12,7	14,4				18,1	19,4
	≥ 65	≥ 55	C40/50	9,7	11,2	12,7	13,7	11,0			15,5	13,8	15,9		
	≥ 76	≥ 65	C50/60												
				Wall t	hicknes	s h = 20	00 mm	Wall t	hicknes	s h = 2	50 mm	Wall thickness h = 300 mm			
	≥ 49	≥ 42	C30/37							28,7	28,7	28,7	28,7	28,7	28,7
	≥ 65	≥ 55	C40/50	16,7	19,3	21,9	23,5	24,8	28,5	32,5	34,8	33,8	34,8	34,8	34,8
	≥ 76	≥ 65	C50/60							ر ۲ ا	54,0	0,00	38,3	38,3	38,3

0

**Note:** At least two Pfeifer VS<sup>®</sup> boxes are to be installed per joint. The transversal shear force carrying capacity perpendicular to the joint may only be applied from a joint length of at least 1.00 m with structural elements with a thickness of less than 0.14 m.

## 6.5 Proof of concept

#### Shear force parallel to the joint

For a shear force parallel to the joint reinforced with the VS<sup>®</sup> Box, a corresponding design resistance (shear force parallel V<sub>Rd,II</sub>) may be applied in the ultimate limit state according to the table above.

#### Shear force perpendicular to the joint

For a shear force perpendicular to the joint reinforced with the VS<sup>®</sup> Box, a corresponding design resistance (shear force perpendicular  $v_{Rd,\perp}$ ) may be applied in the ultimate limit state according to Table 10, depending on the element thickness and the concrete strength class.

Expansion forces (tensile) result from stresses perpendicular to the joint. These tensile forces are absorbed by the wire rope loop.

### Verification of tensile force

Table 11: Verification of tensile force

Stress from	Parallel shear force V <sub>Ed,II</sub>	Perpendicular shear force v <sub>Ed,⊥</sub>	"Outer tensile force"	
Tensile force component		$n_{\text{Ed,VII}} = 0.75 \cdot v_{\text{Ed,II}}  n_{\text{Ed,VII}} = 0.25 \cdot v_{\text{Ed,I}}  n_{\text{Ed,N}}$		
Verification of the total tensile force:	n <sub>Ed,V</sub>	$\frac{11 + n_{Ed,V\perp} + n_{Ed,N}}{n \cdot N_{Rd}} \le$	≤ 1,0	
$\begin{array}{ll} n & [Box/m] \\ N_{Rd} & [kN/Box] \\ n_{Ed,N} & [kN/m] \\ n_{Ed,VII} & [kN/m] \\ \end{array} \\ n_{Ed,VL} & [kN/m] \end{array}$	<ul> <li>Number of VS<sup>®-1</sup></li> <li>Design resistant</li> <li>Acting "outer" te</li> <li>Expansion force metre of joint</li> <li>Expansion force per metre of join</li> </ul>	e – tensile force p ensile force per m from shear force from shear force p	per VS <sup>®</sup> Box hetre of joint parallel per	

#### 6.6 Fire Resistance

Shear forces parallel and

#### perpendicular to the joint combined

When shear forces perpendicular and parallel to the joint act simultaneously, the interaction of the shear forces is to be verified by means of the interaction relationship shown in Table 11.

#### Tensile forces across the VS® loops

The different loading directions result in individual tensile force components that act in the direction of the wire rope loop. The sum of these individual components and any acting "outer" tensile force (total tensile force) is verified on the basis of the tensile force resistance  $N_{Rd}$  of the boxes according to the above table.

#### Verification of shear force

 $\label{eq:VEd,II} \begin{array}{l} V_{Ed,II} \\ \hline V_{Rd,II} \end{array} \leq 1,0 \quad V_{Rd,II} \ [kN/Box]: \mbox{ Acting shear force parallel per box} \\ V_{Rd,II} \ contact \ contact$ 

	$V_{Ed,\perp}$	[kN/m]:	A
$\frac{v_{Ed,\perp}}{v_{Ed,\perp}} \leq 1,0$			р
v <sub>Rd,⊥</sub> ≤1,0	$V_{Rd,\perp}$	[kN/m]:	D
			pe

Acting shear force perpendicular per metre of joint length Design resistance of shear force perpendicular of the joint per metre

 $v_{Ed,II} = n \cdot V_{Ed,II}$ 

 $V_{Ed,II}$  [kN/Box] : acting shear force parallel per box  $v_{Ed,II}$  [kN/m] : acting shear force parallel per metre of joint

According to EN 1992-1-2, section 5.2, the critical temperature for strands and wire ropes is 350 °C. If VS-Boxes are used in load-bearing walls, the design requirements according to 1992-1-2, section 5.4 must be taken into account. According to EN 1992-1-2, sec. 2.1.2 (6) the use of VS-Boxes in fire walls is not permitted if conditions against impact stress (criterion M) must be fulfilled.

# 7. VS<sup>®</sup>-Box selection aid

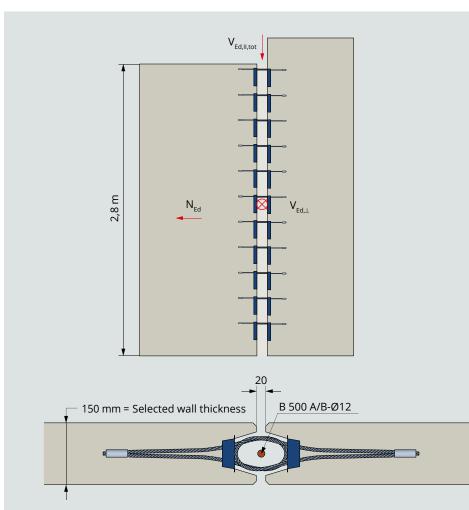
Required input values for selecting the appropriate VS® Box:

- Consistency of the joint filling mortar (free flowing/plastic)
- Joint filling mortar quality
- Structural element concrete quality
- Structural element geometry
- Real load

#### Selection example

- 1. Selection of the appropriate box (criteria: single or double-rope box, mortar free flowing or plastic, etc.)
- 2. Determination of the maximum possible number of boxes
- 3. Table input dimensioning values
- 4. Proof check
- 5. Possible optimisation of the number of boxes with low utilisation

	1			
Joint filling mortar consistency	Thixotropic/plastic			
Choice of box	VS <sup>®</sup> -Box EASYFILL (single-rope box)			
Joint filling mortar quality	C30/37			
Structural element concrete quality	C30/37			
Structural element geometry	Wall thickness: d = 150 mm			
	Wall length: l = 2800 mm			
Real loads	Tensile force: or	N <sub>Ed</sub> = 10 kN n <sub>Ed.N</sub> = 10kN / 2,8 m = 3,6 KN/m		
	Parallel shear force:	$V_{Ed,II,tot} = 50 \text{ kN}$		
	Perpendicular shear force:			
	or	$v_{Ed,\perp}$ = 22,4 kN / 2,8 m = 8,0 kN/m		



Geometric boundary conditions

Wall length [mm]: I<sub>Wall</sub> = 2800

**Box length** [mm]: I<sub>Box</sub> = 160

Min. distance [mm]: s<sub>Box</sub> = 120

**Min. edge distance** [mm]: c<sub>Box</sub> = 180

#### Number of boxes [Box]:

$$\begin{split} n_{\text{Box}} &\leq (I_{\text{Wall}} - 2 \cdot c_{\text{Box}} + s_{\text{Box}}) / \\ (I_{\text{Box}} + s_{\text{Box}}) \\ &= (2800 - 2 \cdot 180 + 120) / \\ (160 + 120) &= 9,14 \end{split}$$

 $n_{Box} = 9$ 

Number of boxes per metre of joint [Box/m]: n = n<sub>Box</sub> / l<sub>Wall</sub> = 9 / 2,8 = 3,2

#### Static proofs

# Extract from Table 8 with resistances $N_{Rd}$ and $V_{Rd,II}$ for VS $^{\circledast}$ Box EASYFILL

	Grout	Precast concrete			
	Compressive strength class			C30/37	
	- [-]	N <sub>Rd</sub> [kN]	V <sub>Rd,II</sub> [kN]	N <sub>Rd</sub> [kN]	V <sub>Rd,II</sub> [kN]
Wall thickness h = 100 mm					
H	C30/37	7.3	11.4	7.3	11.4
Grout	C40/50	9.8	15.0	9.8	15.0
	C50/60	9.8	15.4	10.7	16.6
	Wall thickness h ≥ 120 mm				
	C30/37	7.3	11.4	<sup>3</sup> 7.3	<sup>①</sup> 11.4
	C40/50	9.8	15.0	9.8	15.0
	C50/60	10.1	15.8	12.1	18.3

#### Extract from Table 10 with resistance $v_{Rd, \bot}$ for $VS^{\circledast}\mbox{-Boxes}$

Precast concrete					
V <sub>Rd,L</sub>					
<b>C25/30</b> [kN/m]	<b>C30/37</b> [kN/m]	<b>C35/45</b> [kN/m]	<b>C40/50</b> [kN/m]		
	Wall thickness h = 100 mm				
3,9	4,5	5,2	5,5		
Wall thickness h = 150 mm					
9,7	<sup>©</sup> 11,2	12,7	13,7		

Proof	Parallel shear force V <sub>II</sub>	Perpendicular shear force ${\tt V}_{\! \perp}$	Tensile force N	
Stress total or per box:	$V_{Ed,II,tot} = 50,0 \text{ kN}$ bzw. $V_{Ed,II} = \frac{V_{Ed,II,tot}}{n_{Box}} = \frac{50 \text{ kN}}{9 \text{ Box}} = 5,6 \frac{\text{kN}}{\text{Box}}$	V <sub>Ed,⊥</sub> = 22,4 kN	N <sub>Ed</sub> = 10,0 kN	Stress
Stress per metre of joint:	$v_{Ed,II} = n \cdot V_{Ed,II} = 3,2 \frac{Box}{m} \cdot 5,6 \frac{kN}{Box}$ $= 17,9 \frac{kN}{m}$		$n_{Ed,N} = 3.6 \frac{kN}{m}$	Ś
Resistance per box or per metre of joint:	$V_{Rd,II} = 0.9 \cdot 11.4 \frac{kN}{Box} *)$ $= 10.3 \frac{kN}{Box}$	$v_{Rd,L} = 11,2 \frac{kN **)}{m}$	$N_{Rd} = 0.9 \cdot 7.3 \frac{kN}{Box} *)$ $= 6.6 \frac{kN}{Box}$	Resistance
Proofs:	$\frac{V_{Ed,II}}{V_{Rd,II}} = \frac{5,6 \frac{kN}{Box}}{10,3 \frac{kN}{Box}} = 0,54 \le 1,0$	$\frac{v_{Ed,\perp}}{v_{Rd,\perp}} = \frac{8.0 \frac{kN}{m}}{11.2 \frac{kN}{m}} = 0.71 \le 1.0$	$\frac{0.75 \cdot v_{Ed,II} + 0.25 \cdot v_{Ed,L} + n_{Ed,N}}{n \cdot N_{Rd}} = \le 1.0$ $\frac{0.75 \cdot 17.9 + 0.25 \cdot 8.0 + 3.6}{3.2 \cdot 6.6} = 0.9 \le 1.0$	Proof

\*) Reduction of resistance by 10 % due to use of plastic mortar
 \*\*) No reduction of the resistance necessary with vertical shear force

The proofs in this example are fulfilled with the type VS-Box EASYFILL (single-rope box) selected at the start. If the proofs are not fulfilled, the proof procedure can be repeated with the working load limits of a double-rope box.

#### Caution:

The boxes approved for the respective mortar properties (free flowing or thixotropic plastic) must be taken into account when selecting. These are shown on page 5.



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